DESIGN VALUE
TABLES
**BASE VALUE EQUATIONS**

*These equations are for the use of the Adjustment Factors A through G. Adjustment Factor Tables A through G are for use with the Base Values in Tables 1 and 2.*

For $F_{c/\perp}$ value of 0.02* deformation basis, see Table F.

Note: $C_F = $ Size Factor  
$C_R = $ Repetitive Member Factor  
$C_V = $ Horizontal Shear  
$C_D = $ Duration of Load  
$C_{fu} = $ Flat Use Factor  

**DIMENSION LUMBER ADJUSTMENT FACTORS**

**SIZE FACTORS (C_F)**

Apply to Dimension Lumber Base Values

<table>
<thead>
<tr>
<th>Grades</th>
<th>Nominal Width (depth)</th>
<th>$F_b$</th>
<th>Other Properties</th>
</tr>
</thead>
<tbody>
<tr>
<td>Select Structural, No. 1, No. 2 &amp; No. 3</td>
<td>4” &amp; less</td>
<td>1.5</td>
<td>1.5</td>
</tr>
<tr>
<td></td>
<td>5”</td>
<td>1.4</td>
<td>1.4</td>
</tr>
<tr>
<td></td>
<td>6”</td>
<td>1.3</td>
<td>1.3</td>
</tr>
<tr>
<td></td>
<td>8”</td>
<td>1.2</td>
<td>1.3</td>
</tr>
<tr>
<td></td>
<td>10”</td>
<td>1.1</td>
<td>1.2</td>
</tr>
<tr>
<td></td>
<td>12”</td>
<td>1.0</td>
<td>1.1</td>
</tr>
<tr>
<td></td>
<td>14” &amp; wider</td>
<td>0.9</td>
<td>1.0</td>
</tr>
<tr>
<td>Construction &amp; Standard</td>
<td>4” &amp; less</td>
<td>1.0</td>
<td>1.0</td>
</tr>
<tr>
<td>Utility</td>
<td>2” &amp; 3”</td>
<td>0.4</td>
<td>—</td>
</tr>
<tr>
<td></td>
<td>4”</td>
<td>1.0</td>
<td>1.0</td>
</tr>
<tr>
<td>Stud</td>
<td>4” &amp; less</td>
<td>1.1</td>
<td>1.1</td>
</tr>
<tr>
<td></td>
<td>5” &amp; 6”</td>
<td>1.0</td>
<td>1.0</td>
</tr>
<tr>
<td></td>
<td>8” &amp; wider</td>
<td>use No. 3 grade Base Values and Size Factors</td>
<td></td>
</tr>
</tbody>
</table>
### REPETITIVE MEMBER FACTOR (C<sub>r</sub>)

**TABLE B**

Apply to Size-adjusted F<sub>b</sub>

<table>
<thead>
<tr>
<th>REPETITIVE MEMBER USE</th>
</tr>
</thead>
<tbody>
<tr>
<td>F&lt;sub&gt;b&lt;/sub&gt; x 1.15</td>
</tr>
</tbody>
</table>

Where 2” to 4” thick lumber is used repetitively, such as for joists, studs, rafters and decking, the pieces side by side share the load and the strength of the entire assembly is enhanced. Therefore, where three or more members are adjacent or are not more than 24” apart and are joined by floor, roof or other load distributing elements, the F<sub>b</sub> value can be increased 1.15 for repetitive member use.

### DURATION OF LOAD ADJUSTMENT (C<sub>D</sub>)

**TABLE C**

Apply to Size-adjusted Values

Wood has the property of carrying substantially greater maximum loads for short durations than for long durations of loading. Tabulated design values apply to normal load duration. (Factors do not apply to MOE or F<sub>c⊥</sub>).

<table>
<thead>
<tr>
<th>LOAD DURATION</th>
<th>FACTOR</th>
</tr>
</thead>
<tbody>
<tr>
<td>Permanent</td>
<td>0.9</td>
</tr>
<tr>
<td>Ten Years (Normal Load)</td>
<td>1.0</td>
</tr>
<tr>
<td>Two Months (Snow Load)</td>
<td>1.15</td>
</tr>
<tr>
<td>Seven Day</td>
<td>1.25</td>
</tr>
<tr>
<td>One Day</td>
<td>1.33</td>
</tr>
<tr>
<td>Ten Minutes (Wind and Earthquake Loads)</td>
<td>1.6</td>
</tr>
<tr>
<td>Impact</td>
<td>2.0</td>
</tr>
</tbody>
</table>

Conform load requirements with local codes. Refer to Model Building Codes or the National Design Specification for high-temperature or fire-retardant treated adjustment factors.
HORIZONTAL SHEAR ADJUSTMENT ($C_v$)  
Apply to $F_v$ Values

Horizontal shear values published in Table 1 and 2 are based upon the maximum degree of shake, check or split that might develop in a piece. When the actual size of these characteristics is known, the following adjustments may be taken.

### 2” THICK LUMBER  
3” and THICKER LUMBER

For convenience, the tables below may be used to determine horizontal shear values for any grade of 2” thick lumber in any species when the length of split or check is known:

<table>
<thead>
<tr>
<th>When length of split on wide face is:</th>
<th>Multiply Tabulated $F_v$ value by:</th>
<th>When length of split on wide face is:</th>
<th>Multiply Tabulated $F_v$ value by:</th>
</tr>
</thead>
<tbody>
<tr>
<td>No split</td>
<td>2.00</td>
<td>No split</td>
<td>2.00</td>
</tr>
<tr>
<td>1/2 x wide face</td>
<td>1.67</td>
<td>1/2 x narrow face</td>
<td>1.67</td>
</tr>
<tr>
<td>3/4 x wide face</td>
<td>1.50</td>
<td>1 x narrow face</td>
<td>1.33</td>
</tr>
<tr>
<td>1 x wide face or more</td>
<td>1.00</td>
<td>1 1/2 x narrow or more</td>
<td>1.00</td>
</tr>
</tbody>
</table>

Horizontal shear values for 3” and thicker lumber also are established as if a piece were split full length. When specific lengths of splits are known and any increase in them is not anticipated, the following adjustments may be applied:

### FLAT USE FACTORS ($C_{fu}$)  
Apply to Size-adjusted $F_b$

<table>
<thead>
<tr>
<th>NOMINAL WIDTH</th>
<th>NOMINAL THICKNESS</th>
<th>2” &amp; 3”</th>
<th>4”</th>
</tr>
</thead>
<tbody>
<tr>
<td>2” &amp; 3”</td>
<td>1.00</td>
<td>–</td>
<td></td>
</tr>
<tr>
<td>4”</td>
<td>1.10</td>
<td>1.00</td>
<td></td>
</tr>
<tr>
<td>5”</td>
<td>1.10</td>
<td>1.05</td>
<td></td>
</tr>
<tr>
<td>6”</td>
<td>1.15</td>
<td>1.05</td>
<td></td>
</tr>
<tr>
<td>8”</td>
<td>1.15</td>
<td>1.05</td>
<td></td>
</tr>
<tr>
<td>10” &amp; wider</td>
<td>1.20</td>
<td>1.10</td>
<td></td>
</tr>
</tbody>
</table>
ADJUSTMENTS FOR COMPRESSION PERPENDICULAR TO GRAIN ($F_{C,\perp}$) for Deformation Basis of 0.02"
Apply to $F_{C,\perp}$ Values

Design values for compression perpendicular to grain ($F_{C,\perp}$) are established in accordance with the procedures set forth in ASTM Standards D2555 and D245. ASTM procedures consider deformation under bearing loads as a serviceability limit state comparable to bending deflection because bearing loads rarely cause structural failures. Therefore, ASTM procedures for determining compression perpendicular to grain values are based on a deformation of 0.04” and are considered adequate for most classes of structures. Where most stringent measures need to be taken in design, the following formula permits the designer to adjust design values to a more conservative deformation basis of 0.02”.

$$Y_{02} = 0.73 \times Y_{04} + 5.60$$

EXAMPLE: SPF's $Y_{04} = 335$

$$Y_{02} = 0.73 \times (335) + 5.60 = 250 \text{ Psi}$$

WET USE FACTORS ($C_M$)
Apply to Size-adjusted Values

The recommended design values shown in the accompanying tables are for applications where the moisture content of the wood does not exceed 19%. For use conditions where the moisture content of dimension lumber will exceed 19%, the Wet Use Adjustment Factors below are recommended:

<table>
<thead>
<tr>
<th>PROPERTY</th>
<th>ADJUSTMENT FACTOR</th>
</tr>
</thead>
<tbody>
<tr>
<td>$F_b$ Extreme Fiber Stress in Bending</td>
<td>0.85*</td>
</tr>
<tr>
<td>$F_t$ Tension Parallel to Grain</td>
<td>1.0*</td>
</tr>
<tr>
<td>$F_{C/\parallel}$ Compression Parallel to Grain</td>
<td>0.8**</td>
</tr>
<tr>
<td>$F_v$ Horizontal Shear</td>
<td>0.97</td>
</tr>
<tr>
<td>$F_{C,\perp}$ Compression Perpendicular to Grain</td>
<td>0.67</td>
</tr>
<tr>
<td>$E$ Modulus of Elasticity</td>
<td>0.9</td>
</tr>
</tbody>
</table>

* Fiber Stress in Bending Wet Use Factor 1.0 for size-adjusted $F_b$ not exceeding 1150 psi.
** Compression Parallel to Grain in Wet Use Factor 1.0 for size-adjusted $F_c$ not exceeding 750 psi.
### TABLE 1

**DIMENSION LUMBER BASE VALUES — SOFTWOODS**

2” to 4” THICK by 2” and WIDER

USE WITH ADJUSTMENT TABLES A THROUGH G

Design Values in Pounds Per Square Inch

Grades Described in para. 10.0-12.4

Also Stress Rated Boards, See para. 17.0

<table>
<thead>
<tr>
<th>Species or Group</th>
<th>Grade</th>
<th>Extreme Fiber Stress in Bending “Fb”</th>
<th>Tension Parallel to Grain “Ft”</th>
<th>Horizontal Shear “Fv”</th>
<th>Compression Perpendicular “Fc⊥”</th>
<th>Compression Parallel to Grain “Fc∥”</th>
<th>Modulus Elasticity “E”</th>
</tr>
</thead>
<tbody>
<tr>
<td>SPF(s)** (Eastern Spruce, Norway Spruce, Red Pine, Jack Pine, Balsam Fir)</td>
<td>Sel. Str.</td>
<td>1300</td>
<td>575</td>
<td>135</td>
<td>335</td>
<td>1200</td>
<td>1,300,000</td>
</tr>
<tr>
<td></td>
<td>No. 1</td>
<td>875</td>
<td>400</td>
<td>135</td>
<td>335</td>
<td>1050</td>
<td>1,200,000</td>
</tr>
<tr>
<td></td>
<td>No. 2</td>
<td>775</td>
<td>350</td>
<td>135</td>
<td>335</td>
<td>1000</td>
<td>1,100,000</td>
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<tr>
<td></td>
<td>No. 3</td>
<td>450</td>
<td>200</td>
<td>135</td>
<td>335</td>
<td>575</td>
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<tr>
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<td>Construction</td>
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<td>400</td>
<td>135</td>
<td>335</td>
<td>1200</td>
<td>1,000,000</td>
</tr>
<tr>
<td></td>
<td>Standard</td>
<td>500</td>
<td>225</td>
<td>135</td>
<td>335</td>
<td>1000</td>
<td>900,000</td>
</tr>
<tr>
<td></td>
<td>Utility</td>
<td>225</td>
<td>100</td>
<td>135</td>
<td>335</td>
<td>675</td>
<td>900,000</td>
</tr>
<tr>
<td></td>
<td>Stud</td>
<td>600</td>
<td>275</td>
<td>135</td>
<td>335</td>
<td>625</td>
<td>1,000,000</td>
</tr>
<tr>
<td></td>
<td>Sel. Str.</td>
<td>No. 1</td>
<td>No. 2</td>
<td>No. 3</td>
<td>Construction</td>
<td>Standard</td>
<td>Utility</td>
</tr>
<tr>
<td>----------------</td>
<td>----------</td>
<td>-------</td>
<td>-------</td>
<td>-------</td>
<td>--------------</td>
<td>----------</td>
<td>---------</td>
</tr>
<tr>
<td>EASTERN Sel. Str.</td>
<td>1250</td>
<td>775</td>
<td>575</td>
<td>350</td>
<td>675</td>
<td>375</td>
<td>175</td>
</tr>
<tr>
<td>WHITE PINE</td>
<td>575</td>
<td>275</td>
<td>150</td>
<td>300</td>
<td>175</td>
<td>75</td>
<td>200</td>
</tr>
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<td>135</td>
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</tr>
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<td>350</td>
<td>350</td>
<td>350</td>
<td>350</td>
<td>350</td>
</tr>
<tr>
<td>No. 3</td>
<td>1200</td>
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<td>825</td>
<td>475</td>
<td>1050</td>
<td>850</td>
<td>550</td>
</tr>
<tr>
<td>Construction</td>
<td>1,200,000</td>
<td>1,100,000</td>
<td>1,100,000</td>
<td>900,000</td>
<td>1,000,000</td>
<td>900,000</td>
<td>800,000</td>
</tr>
<tr>
<td>Standard</td>
<td>1,100,000</td>
<td>1,100,000</td>
<td>1,100,000</td>
<td>900,000</td>
<td>1,000,000</td>
<td>900,000</td>
<td>800,000</td>
</tr>
<tr>
<td>Utility</td>
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<td>1,100,000</td>
<td>1,100,000</td>
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<td>1,000,000</td>
<td>900,000</td>
<td>800,000</td>
</tr>
<tr>
<td>Stud</td>
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<td>1,100,000</td>
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<td>1,000,000</td>
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<td>800,000</td>
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</tbody>
</table>

<table>
<thead>
<tr>
<th></th>
<th>Sel. Str.</th>
<th>No. 1</th>
<th>No. 2</th>
<th>No. 3</th>
<th>Construction</th>
<th>Standard</th>
<th>Utility</th>
<th>Stud</th>
</tr>
</thead>
<tbody>
<tr>
<td>NORTHERN Sel. Str.</td>
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<td>325</td>
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<td>325</td>
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<td>250</td>
</tr>
<tr>
<td>CEDAR</td>
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<td>325</td>
<td>300</td>
<td>200</td>
<td>100</td>
<td>250</td>
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<td>120</td>
<td>120</td>
</tr>
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<td>No. 3</td>
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<td>275</td>
<td>625</td>
<td>475</td>
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<tr>
<td>Construction</td>
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<td>600,000</td>
<td>700,000</td>
<td>600,000</td>
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</tr>
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<td>700,000</td>
<td>600,000</td>
<td>600,000</td>
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</tr>
<tr>
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<td>700,000</td>
<td>700,000</td>
<td>600,000</td>
<td>700,000</td>
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<tr>
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<td>600,000</td>
<td>700,000</td>
<td>600,000</td>
<td>600,000</td>
<td>600,000</td>
</tr>
</tbody>
</table>
### TABLE 1
DIMENSION LUMBER BASE VALUES — SOFTWOODS

2” to 4” THICK by 2” and WIDER

USE WITH ADJUSTMENT TABLES A THROUGH G

Design Values in Pounds Per Square Inch

Grades Described in para. 10.0-12.4

Also Stress Rated Boards, See para. 17.0

<table>
<thead>
<tr>
<th>Species or Group</th>
<th>Grade</th>
<th>Extreme Fiber Stress in Bending “Fb” in Single Member</th>
<th>Tension Parallel to Grain “Ft”</th>
<th>Horizontal Shear “Fv”</th>
<th>Compression Perpendicular “Fc⊥”</th>
<th>Compression Parallel to Grain “Fc//”</th>
<th>Modulus Elasticity “E”</th>
</tr>
</thead>
<tbody>
<tr>
<td>EASTERN Sel. Str.</td>
<td>1250</td>
<td></td>
<td>575</td>
<td>170</td>
<td>555</td>
<td>1200</td>
<td>1,200,000</td>
</tr>
<tr>
<td>No. 1</td>
<td>775</td>
<td></td>
<td>350</td>
<td>170</td>
<td>555</td>
<td>1000</td>
<td>1,100,000</td>
</tr>
<tr>
<td>No. 2</td>
<td>575</td>
<td></td>
<td>275</td>
<td>170</td>
<td>555</td>
<td>825</td>
<td>1,100,000</td>
</tr>
<tr>
<td>No. 3</td>
<td>350</td>
<td></td>
<td>150</td>
<td>170</td>
<td>555</td>
<td>475</td>
<td>900,000</td>
</tr>
<tr>
<td>Construction</td>
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<td>170</td>
<td>555</td>
<td>1050</td>
<td>900,000</td>
</tr>
<tr>
<td>Standard</td>
<td>375</td>
<td></td>
<td>175</td>
<td>170</td>
<td>555</td>
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<td>800,000</td>
</tr>
<tr>
<td>Utility</td>
<td>175</td>
<td></td>
<td>75</td>
<td>170</td>
<td>555</td>
<td>550</td>
<td>800,000</td>
</tr>
<tr>
<td>Stud</td>
<td>450</td>
<td></td>
<td>200</td>
<td>170</td>
<td>555</td>
<td>525</td>
<td>900,000</td>
</tr>
<tr>
<td>HEMLOCK- Tamarack**</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>No. 1</td>
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<td></td>
<td>350</td>
<td>170</td>
<td>555</td>
<td>1000</td>
<td>1,100,000</td>
</tr>
<tr>
<td>No. 2</td>
<td>575</td>
<td></td>
<td>275</td>
<td>170</td>
<td>555</td>
<td>825</td>
<td>1,100,000</td>
</tr>
<tr>
<td>No. 3</td>
<td>350</td>
<td></td>
<td>150</td>
<td>170</td>
<td>555</td>
<td>475</td>
<td>900,000</td>
</tr>
<tr>
<td>Construction</td>
<td>675</td>
<td></td>
<td>300</td>
<td>170</td>
<td>555</td>
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<td>900,000</td>
</tr>
<tr>
<td>Standard</td>
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<td></td>
<td>175</td>
<td>170</td>
<td>555</td>
<td>850</td>
<td>800,000</td>
</tr>
<tr>
<td>Utility</td>
<td>175</td>
<td></td>
<td>75</td>
<td>170</td>
<td>555</td>
<td>550</td>
<td>800,000</td>
</tr>
<tr>
<td>Stud</td>
<td>450</td>
<td></td>
<td>200</td>
<td>170</td>
<td>555</td>
<td>525</td>
<td>900,000</td>
</tr>
<tr>
<td>TAMARACK**</td>
<td>No. 2</td>
<td></td>
<td>575</td>
<td>170</td>
<td>555</td>
<td>825</td>
<td>1,100,000</td>
</tr>
<tr>
<td></td>
<td>No. 3</td>
<td></td>
<td>350</td>
<td>170</td>
<td>555</td>
<td>475</td>
<td>900,000</td>
</tr>
<tr>
<td></td>
<td>Construction</td>
<td></td>
<td>675</td>
<td>170</td>
<td>555</td>
<td>1050</td>
<td>900,000</td>
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* See para. 31.0 through 37.0 for additional information on these values.
** The same values apply to each species if stamped individually.
*** Eastern Softwoods species grouping = Any combination of the above species, excluding Northern White Cedar.
TABLE 2  
DIMENSION LUMBER BASE VALUES — HARDWOODS

2” to 4” THICK by 2” and WIDER
USE WITH ADJUSTMENT TABLES A THROUGH G

Design Values in Pounds Per Square Inch

Grades Described in para. 10.0-12.4 
Also Stress Rated Boards, See para. 17.0

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<th>Species or Group</th>
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<th>Extreme Fiber Stress in Bending “Fb”</th>
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DIMENSION LUMBER BASE VALUES — HARDWOODS

2” to 4” THICK by 2” and WIDER
USE WITH ADJUSTMENT TABLES A THROUGH G

Design Values in Pounds Per Square Inch

Grades Described in para. 10.0-12.4
Also Stress Rated Boards, See para. 17.0

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TABLE 3

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Design Values in Pounds Per Square Inch

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### TABLE 3 (Continued)

BEAMS and STRINGERS (5” and THICKER)

WIDTH MORE THAN 2” GREATER THAN THICKNESS

Design Values in Pounds Per Square Inch

Grades Described in para. 25.0-25.4

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**TABLE 4**

**POSTS and TIMBERS (5” X 5” and LARGER)**

*WIDTH NOT MORE THAN 2” GREATER THAN THICKNESS*

*Design Values in Pounds Per Square Inch*

Grades Described in para. 26.0-26.3

<table>
<thead>
<tr>
<th>Species or Group</th>
<th>Grade</th>
<th>Extreme Fiber Stress in Bending “Fb”</th>
<th>Tension Parallel to Grain “Ft”</th>
<th>Horizontal Shear “Fv”</th>
<th>Compression Perpendicular “Fc⊥”</th>
<th>Compression Parallel to Grain “Fc∥”</th>
<th>Modulus Elasticity “E”</th>
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### TABLE 4
(Continued)

**POSTS and TIMBERS (5” X 5” and LARGER)**

WIDTH NOT MORE THAN 2” GREATER THAN THICKNESS

Design Values in Pounds Per Square Inch

Grades Described in para. 26.0-26.3

<table>
<thead>
<tr>
<th>Species or Group</th>
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<th>Horizontal Shear “Fv”</th>
<th>Compression Perpendicular “Fc⊥”</th>
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